

# Buckling of Composite Beams (CDDF Final Report—Project No. 91–20)

P. Thompson



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## LIST OF SYMBOLS

b	beam width, inches
h	beam height, inches
$t_w$	web thickness, inches
$t_f$	flange thickness, inches
P	load applied, lb
E	modulus of elasticity, lb/in <sup>2</sup>
d	deflection, inches
$\phi$	angle of twist, degrees
Θ	ply orientation angle, degrees
m,n	laminate stacking sequence number

#### TECHNICAL PAPER

#### BUCKLING OF COMPOSITE BEAMS

#### INTRODUCTION

Torsion/bending coupling is an interesting, inherent phenomenon occurring in open-section structural elements subjected to bending and/or combined axial/bending loads. The benefits of using open-section beams are many: stackability, weight savings, and dual functions (e.g., serving as load carriers as well as conduits). Likewise, composites offer advantages over their metallic counterparts; primarily, their higher strength-to-weight ratios and lower coefficients of thermal expansion (CTE) are favorable. This type beam is used extensively in the aircraft industry in the fuselage support structure and wing supports, and has present and potential future use in aerospace applications; namely tankage stiffeners, intertank stiffeners, and as primary load carriers in truss work. However, in each of the aforementioned applications the layups are symmetric in nature. By understanding the bending/torsion effects indigenous to open-section beams one can make use of the tailorability of composites (to achieve desired optimum material properties) without compromise to the design (e.g., no added plies to accomplish symmetry).

Antisymmetric layups lend themselves well to thin-walled structural members. With an average ply thickness of 0,005 in, a beam of wall thickness less than 0,040 in (8 plies) is difficult to manufacture using a symmetric laminate layup.

Even an open-section beam made of an isotropic material has an inherent tendency to bend and twist if the transverse load is applied anywhere other than along the plane of the shear center. A transverse load applied to such open sections undergoes pure bending only when the load is applied at points along the shear center located outside the section. As Valsov illustrates, an open section not loaded at points along its shear center will twist and bend as in figure 1(b). Figure 1(a) shows the same open-section beam with no load applied. In the case with loading at the shear center, notice that pure bending results, as in figure 1(c).

As in symmetrically laminated structural elements, unsymmetrically laminated elements can have many different resultant properties depending on the chosen material system, ply orientations, and layup sequence. Add these factors to the bending/twisting coupling inherent to an open section loaded transversely and one can easily surmise the difficulties that might be associated with designing using unsymmetric, open-section structural elements.

Characterization of the relationship between angle of twist and ply orientation is one of the objectives of this paper. This is being done based strictly on test results, with results of the analytical aspect to be presented in a later paper. Also examined and presented here are the results of testing involving shear center location, load application relative to shear center, and the resulting angle of twist on open-section beams. Curves depicting these parameters and the various interrelationships are contained in this paper.

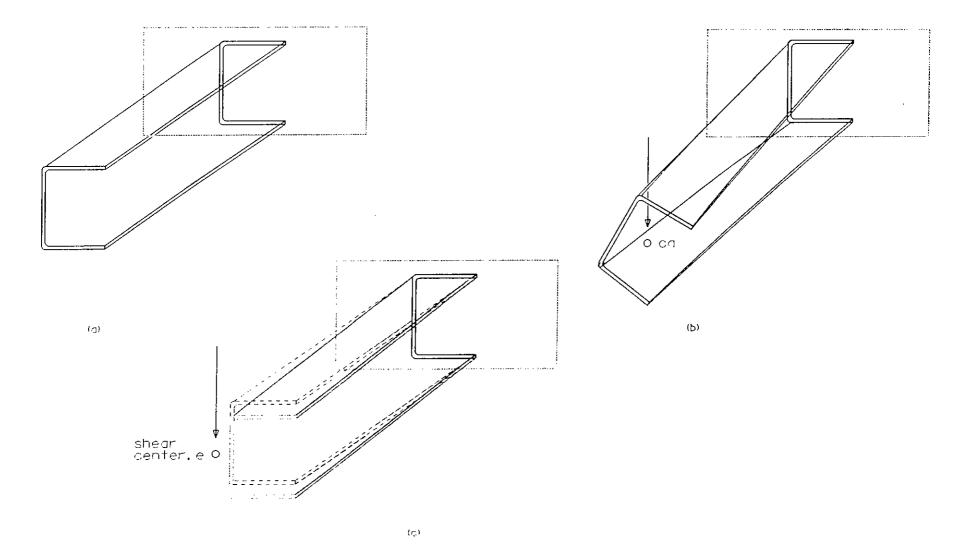


Figure 1. Open-section beam.

#### APPROACH

To reach the first objective (characterization of the relationship between the angle of twist and ply orientation), the effects of distortions due to CTE must be included. This produces an amount of pretwist in the beam apart from the twisting that is to occur due to the load not being applied at the actual shear center. Examination of the various laminate layups will reveal a general relationship between the ply orientation and angle of twist.

Open-section beams of various ply orientations were manufactured and tested to determine what effect ply orientation and laminate layup have on the shear center location, i.e., can the shear center in open-section structural elements be shifted toward its geometric center? As illustrated in figure 2, the geometric center and theoretical shear center differ. Yet this moving of the shear center can possibly be observed by comparing the various twist and deflections in each beam tested as a specified load is applied at designated positions along the horizontal plane of the shear center (fig. 2). Recalling that no twist (pure bending only) will occur when transverse loads are applied at the shear center, one can better define the location of the "true" shear center. This "true" position, since it is observed after the curing process (hence, CTE mismatches are already accounted for), reflects the dependency of shear center on laminate layup. No attempts were made to quantify this preload twist; however, measurements were taken prior to testing the beams to analyze trends.

Open-section beams (C-channels) of the layup  $[0/\Theta]_n$  are referred to as Form 1. The laminates referred to as Form 2 are of  $([0/\Theta]_n, [0/\Theta]_n)$  construction.

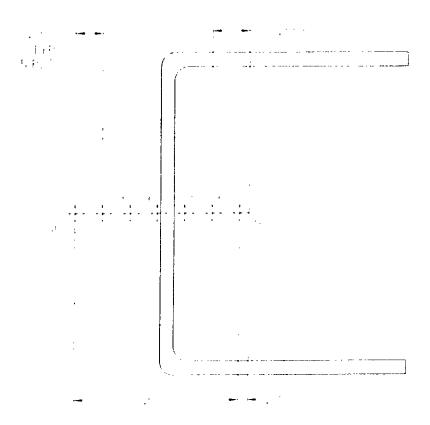


Figure 2. Load application points.

#### MANUFACTURING METHODS AND TESTING

As previously stated, the beams were fabricated using the hand layup and hot-drape forming techniques. Hand layup involved cutting 3-in wide strips of the unidirectional tape to the correct lengths and wrapping the tape at proper angles across the tool (in this case a solid aluminum square tube served as the tool) layer by layer. Intermediate vacuum bagging was performed to ensure adequate compaction prior to the autoclave cure of the part. Hot-drape forming was a new procedure used to take advantage of the multiaxis tape laying machine. Using this technique panels (approximately 18-in wide by 72-in long) were tape layed by machine. This provided repeatability of the part, provided adequate compaction (tape head is pressure sensitive, i.e., constant pressure as tape is layed in place), and reduced time of manufacturing. After the panels were cut to a strip 9 by 72 in, they were "draped" over the same tool used previously, bagged, and placed in the hot-drape forming "oven" under controlled temperatures (see the photographs in the appendix). This allowed the material's resin to flow enough so that the panel would take the shape of the tool.

After the part was formed (regardless of method), it was vacuum bagged for autoclave cure using standard procedures. The resulting channels were then removed from the tool and cut and machined to dimensions suitable for testing. Figure 3 shows the geometry of the finished beam prior to instrumentation for testing.

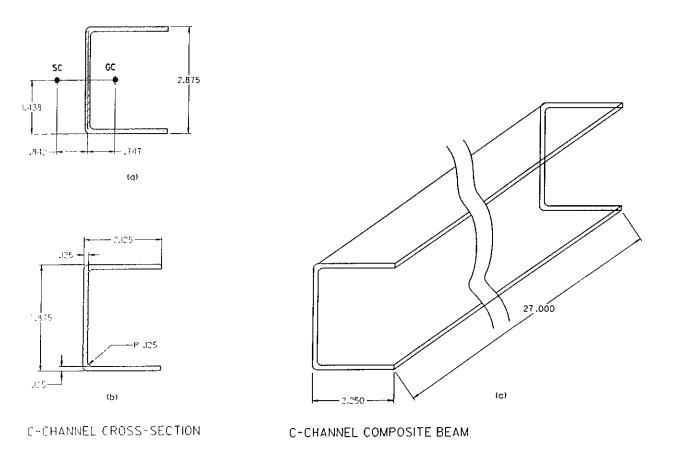


Figure 3. C-channel beams.

The testing included applying load at several locations along the horizontal plane of the shear and geometric centers. After the beam was instrumented and the load cells calibrated, load was applied in 15 to 20 lb increments until a total of 130 lb was being applied. Beginning at the theoretical shear center, this procedure was repeated (at 0.25-in increments) until the final measurements at the geometric center (fig. 2). During the test procedure, the deflections and strains were recorded with the addition of load at each of the incremental points.

#### EFFECT OF FIBER ORIENTATION OF FLANGE DEFLECTION

Graphs 1 through 6 of section I of the appendix show deflections in the upper and lower flanges for samples of various layups of the C-channels. Also, charts 1 through 6 show a complete set of deflection data for the  $[(0/75)]_{12}$  case. The beams were instrumented as shown in figure 4. An interesting occurrence in the beams, discovered through observation of the test results, is that the deflections decreased as the load application point is moved away from the center of gravity. This fact becomes more important in the following section when angle of twist is discussed. As expected, the deflection represented by D9 on the charts is smallest; this measurement is taken at a point on the beam's web. The top flange deflected the most in all cases examined.

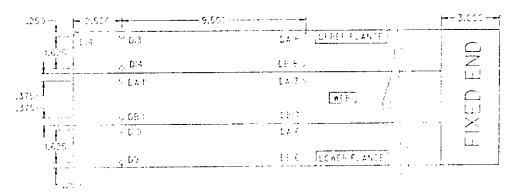


Figure 4. Deflection gauge locations for composite beam load tests.

Another observation was that for beams having the same ply angle, the beams of Form 1 deflected at least twice (100 percent) as much as those of Form 2 when the load was applied near the shear center, and almost 80 percent more when loaded at the geometric center. Additionally, by examining the deflection data at the 0.25-in increments, more interesting facts were revealed. For example, in the case where the cross ply angle is  $\pm 75^{\circ}$ , it is noted that the deflection in the web and flanges decrease (for the maximum load case) as one moves toward the calculated shear center located 1.44 in from the geometric center. Yet, as will be discussed later, the deflections approach zero in the web even more by moving further than 1.44 in from the shear center. This serves as an indicator that the true shear center is further than 1.44 in away from the centroid. Therefore, one would opt for the laminate layup of Form 2 in situations in which low deflections are desired.

Two materials systems were used: AS4/3501-6 and IM7/8552. Figures 5 through 8 show deflections measured at the geometric centers and shear centers for a [0/75]12 beam of each material. Due to material properties, the IM7/8552 beams deflected more than the AS4/3501-6 beams. Another point of interest from the data gathered is that the further away from the geometric center that the load is applied, the greater the difference in the deflection amounts. The explanation of this could also form the basis for more research work.

# AS4/3501-6 [0/75]12

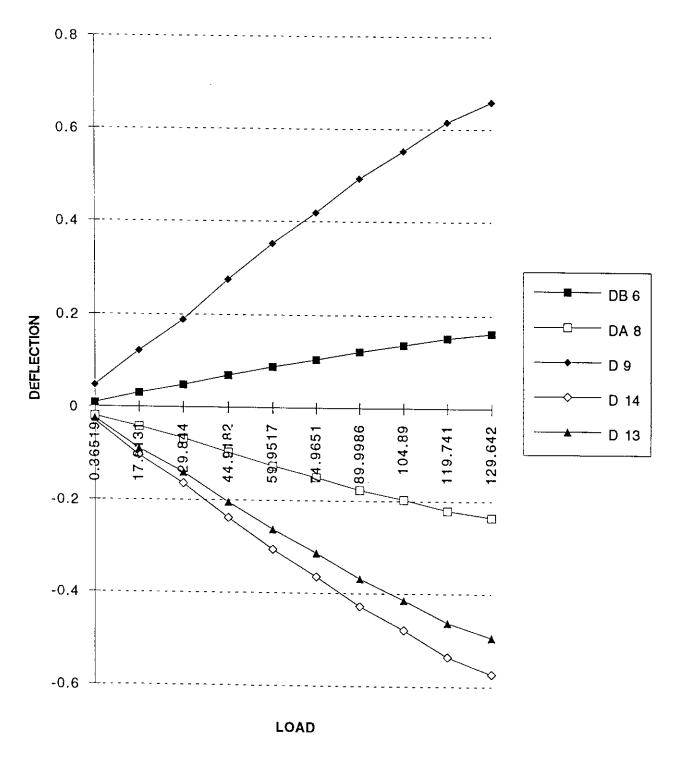


Figure 5. Deflections for C-channel loaded at geometric center (AS4/3501-6).

# AS4/3501-6 [0/75]12

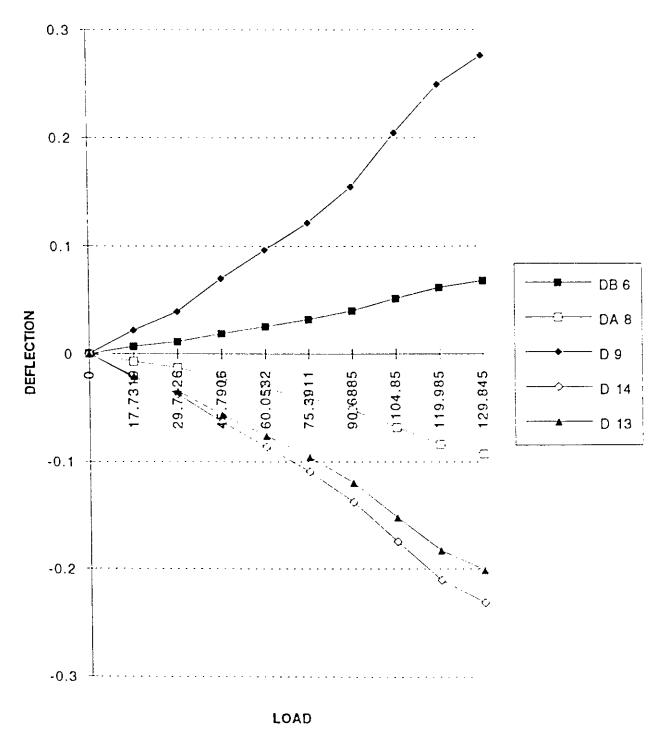


Figure 6. Deflections for C-channel loaded at theoretical shear center (AS4/3501-6).

## IM7/8552 [0/75]12

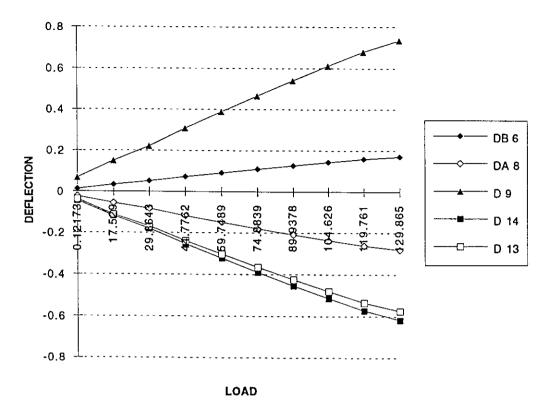


Figure 7. Deflections for C-channel loaded at geometric center (IM7/8552).

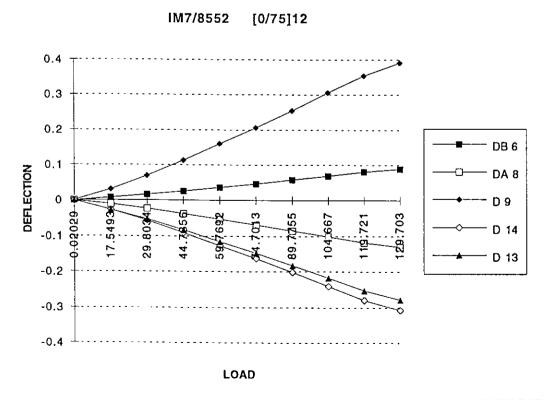


Figure 8. Deflections for C-channel loaded at theoretical shear center (IM7/8552).

#### EFFECT OF FIBER ORIENTATION OF SHEAR CENTER LOCATION

For this research effort, the theoretical shear center for the given cross section is easily determined by the formula:

$$e = bht/41$$
.

After this shear center is known, one would expect that a transverse load applied through this point would produce only bending, and that the same load applied through the beam's geometric center produces both bending and torsion coupling. Yet, due to the layup of these open-section members (no midplane symmetry) the true shear center does not follow this equation. The research work showed that for some of the laminate layups the true shear center actually lies between the geometric center and theoretical shear center, and could, in some cases, lie beyond the calculated shear center. As the angle increased, the amount of twist observed and measured when the beam was loaded at this "predicted" shear center increased also. As the load application point was shifted toward the center of gravity (e.g.), the trend was for more twisting to occur. However, for most of the cases tested, this increase did not occur until after the load application point had been shifted more than 0.25 in from the shear center in the direction of the e.g. This would indicate that the true shear center is actually between the predicted shear center and a 0.25 in toward the e.g. Yet, for those beams of Form 1 with a cross-ply angle greater than 45°, the "true" shear center was outside the theoretical value, i.e., more than 1.44 in from the geometric center.

Figure 9 shows the angle of twist for various layups as calculated by the formulas<sup>3</sup>

$$e = d_1/2L$$
 and  $e = d_2/2L$ ,

$$\phi = d_1/(h/2)$$
 and  $\phi = d_2/(h/2)$ ,

where

h = height of web

b =width of flange

 $d_1$  = web deflection

 $d_2$  = flange deflection.

For presentation purposes, beams made with only the  $15^{\circ}$ ,  $45^{\circ}$ , and  $75^{\circ}$  cross-ply angles of Form 1 are shown. Measured strains and deflections for the  $30^{\circ}$  and  $60^{\circ}$  cases fell between the extremes of the charts shown. Charts 1 through 6 in section II of the appendix contain the strain data as measured for the  $[(0/15)]_{12}$  beam.

Graphically, one can see from figure 10 that the angle of twist does increase with an increase in the cross-ply laminate angle, regardless of the overall layup (Form 1 or Form 2). The range in angle of twist as measured is between -18° and 55° for Form 1, while its range was -25° to 75° for Form 2 beams.

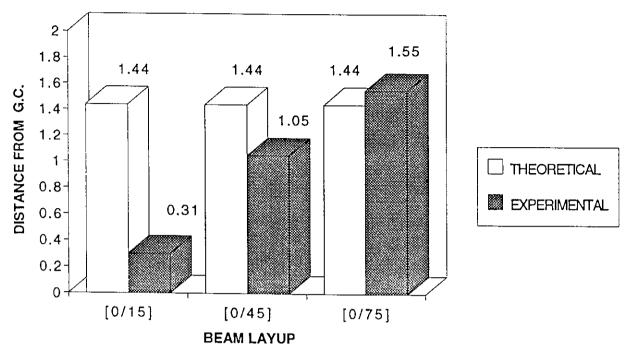


Figure 9. Theoretical versus measured shear center comparison.

What these equations and formulas do not show, however, is the postcure inward canting of the beams as shown in figure 7. No quantitative assessment of the amount of this cant is presented here. Nevertheless, it should be pointed out that this phenomena occurred in all the beams. Interestingly, the beams containing 45° plies of Form 1 canted outward (i.e., pulled away from the tool after curing) and twisted along the length of the beam by more than 90°. CTE's, as well as interlaminar shear stress and strains during the curing process, would have to be closely examined. The explanation of this occurrence could provide the basis for additional research work.

Test data were also used to gain insight on the location of the beam's "true" shear center (i.e., no twist when transverse load applied) as a function of laminate layup. Due to the beams' geometry, they are expected to twist as well as bend, and all the beams tested experienced this. However, the focus here was to attempt to categorize the amount of twist that could be expected from a given layup. In analyzing the test results, several interesting trends were noticed and are reported here.

First, the beams of the Form 1 construction had higher degrees of rotation (angles of twist) than those of Form 2 for any given cross-ply lamina. Remembering that the Form 1 beams have cross plies only in the positive direction, whereas the Form 2 beams have both positive and negative plies, it is obvious that, upon completion of the appropriate stiffness matrices, this in itself is not abnormal.

Figure 10 reveals another trend. By calculations using the given geometry of the beams, one can find that the theoretical shear center is located 1.44 in to the left of the geometric center as shown in figure 3. By observing the charts, one can see that none of the plotted beams show an experimental shear center that matches its theoretical value. All the beams were of the same overall geometry and the results shown on the charts were based on a 130-lb transverse load placed on the beams.

#### [0/15] Laminate Layup Angle of Twist 60 ± 50 40 Angle of Twist 30 20 10 G.C. 0 0.19 0.44 1.19 1.44 0.69 0.94 -10 -20 [0/45] Laminate Layup Angle of Twist 10 5 S.C. Angle of Twist G.C. 0 1.44 0.94 1.19 0.19 0.44 0.69 -5 -10 -15 -20 <sup>±</sup> [0/75] Laminate Layup Angle of Twist

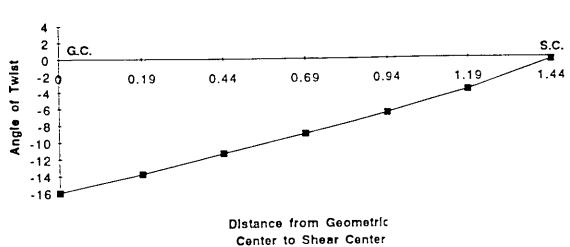


Figure 10. Angle of twist versus load application points for Form 1 beam (AS4/3501-6).

The  $[0/75]_{12}$  layup most closely approached the theoretical value and had the highest angle of twist, while the  $[0/15]_{12}$  layup had the lowest angle of twist. This tendency, particularly the least twist in the  $[0/15]_{12}$  members, was not expected. It would appear intuitively that since the tendency to rotate in these beams would not be offset by the almost horizontal plies of the 15° laminates they would have the higher twist angle. Similarly, these same beams had true shear centers located more than 1 inch from the theoretical value.

All beams had some small amount of twist before loading due to CTE differences between the plies during the cure process. This pretwist was most noticeable in the  $[0/45]_{12}$  beams which rotated more than 90° along the length of the beam. Pretwist due to CTE mismatches is not addressed here, but it is worthy of future consideration, as is controlling the various parameters comprising the cure cycle (e.g., cure pressure, tooling materials, cure temperature profiles). This pretwist, however, does not significantly alter the trends reported here since the loads were applied at the calculated theoretical shear center and additional twisting occurred, again verifying the shifting of the "true" shear center. Figures 11(a) and (b) shows the angle of twist for Form 1 beams when  $\Theta$  is equal to 15, 45, and 75, respectively, and only a minimal load is applied (P < 5 lb).

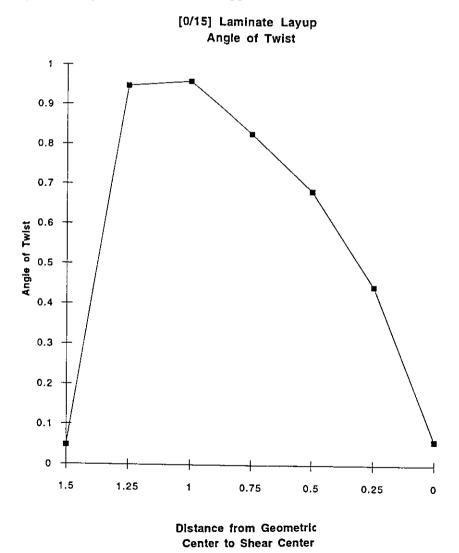


Figure 11(a). Angle of twist versus load application points for Form 1 beam with minimal load applied,  $P \le 5.0$  lb (AS4/3501-6).

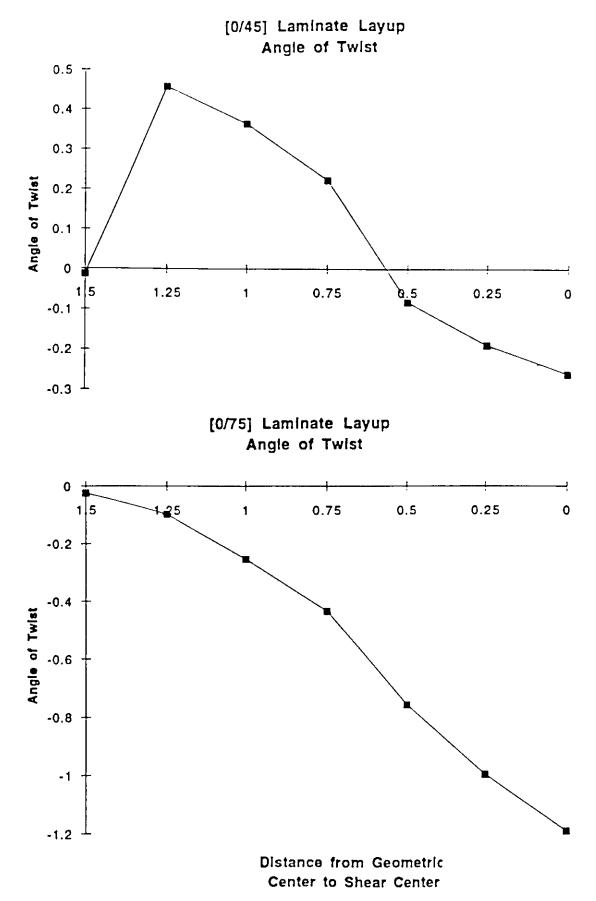


Figure 11(b). Angle of twist versus load application points for Form 1 beam with minimal load applied,  $P \le 5.0$  lb (AS4/3501-6).

Assuming plate theory, it can be shown that in-plane stress resultants for laminated structures are not only functions of midplane strains but can be functions of curvatures and twists. Additionally, the in-plane forces can cause deformations that can cause curvatures or twisting deformations.<sup>4</sup> Plate theory does not directly apply to beams as presented in the above cases. As Vinson points out, the above analytical procedures are valid only if no coupling exists, i.e., the  $D_{16}$  and  $D_{26}$  terms of the stiffness matrix are zero.<sup>5</sup> The equations were used in this paper only to produce a relative number that would allow numerical comparisons to be made. More complex analysis that considers not only the nonzero cases for  $D_{16}$  and  $D_{26}$  but also hygrothermal effects would need to be developed to more accurately assess the true values for the amount of twist produced due to laminate angle orientation.

Even though plate theory may not be directly applicable to beam theory, one can use it to gather trend data. This is an attempt to categorize the behavior of the laminates rather than assign a quantitative value for the angle of twist. Regardless of the number assigned to this angle, the observed trends would still be valid. The analysis method used to arrive at this value does not influence the direction or importance of the trend.

#### **CONCLUSIONS**

The magnitude of information gathered through tests performed on open-section unsymmetric beams was tremendous. All data gathered were not discussed in this paper; however, subsequent writings will explore other areas worthy of further investigation in an attempt to more accurately categorize the behavior of such beams. Major findings reported here deal with the primary objective regarding shear center location and manipulation, as well as angle of twist and warpage in these structural members.

One important revelation is that the angles of twist and associated warpage in open-section beams is higher for antisymmetric beams of the  $[0/\phi]m$  laminate layup. Some amount of bending/stretching is expected in any antisymmetric layup due to the fact that the components of the [B] coupling matrix, which relates stress couples to midsurface planes and stress resultants to curvatures, are not all zero if the structure is not symmetric about its midplane. Likewise, unless the D16 and D26 terms of the stiffness matrix [D], which relates the stress couples to curvatures, are zero, bending/twisting will occur. No attempt is made to measure the magnitude of hygrothermal effects on twist in the beam, even though there could be as many as three different CTE's depending on cross-ply angle and orientation, all which play a role in the warping of the beams.

The major fact borne out here is that laminate layup does control shear center location. As evidenced by charts presented earlier, the same geometric shape can yield "true" shear centers that differ by more than an inch depending on laminate layup. The location of the "true" shear center for the type beam investigated here depends on several factors. Cross-ply angle sequence is important (Form 1 or Form 2) in that other unsymmetrical layups using the same angle would yield different shear center locations. Yet not to be overlooked are factors such as cure cycle and laminate materials.

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- 2. Nemeth, M.P.: "Buckling Behavior of Long Symmetrically Laminated Plates Subjected to Combined Loadings." LaRC, NASA Technical Paper 3195.
- 3. Vinson, J.R., and Serakowski, R.L.: "The Behavior of Structures Composed of Composite Materials." Martinus Nijhoff Publisher, 1986, pp. 101–103.
- 4. Ibid., p. 54.
- 5. Ibid., p. 82.
- 6. Ibid., p. 56.
- 7. Ibid. p. 41.

## **APPENDIX**

### **Contents**

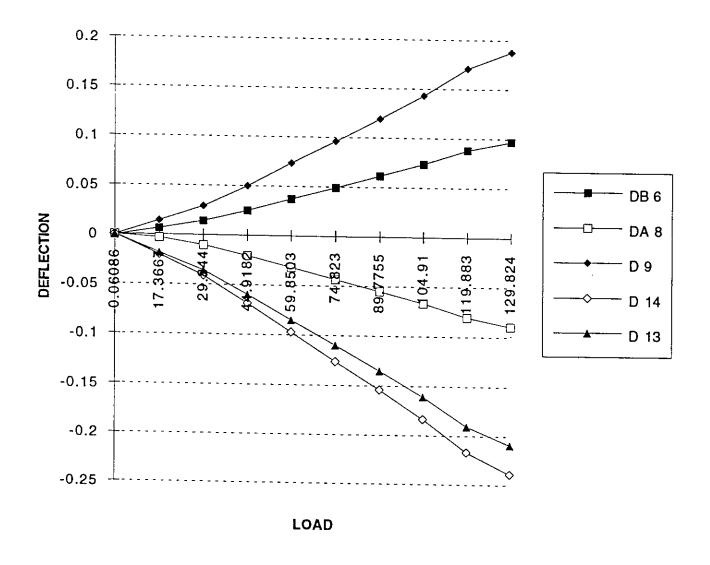
- I. Deflection Graphs and Data
- II. Test Strain Data
- III. Manufacturing and Testing Photographs

## SECTION I

Deflection Graphs and Data

## IM7/8552 1.5 IN GC GRAPH

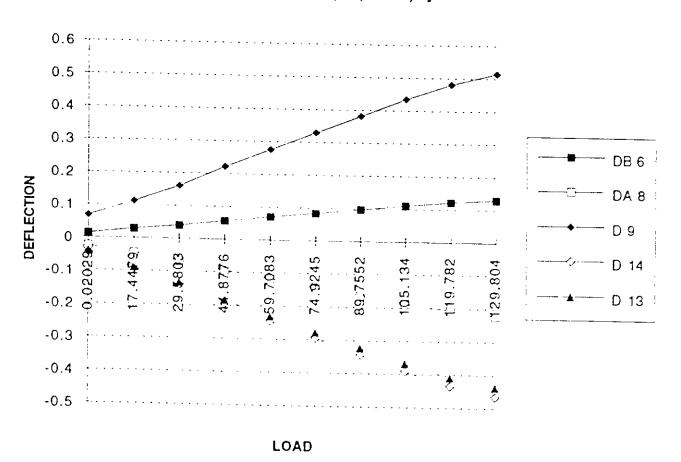
# IM7/8552 [0/15]12



Graph 1

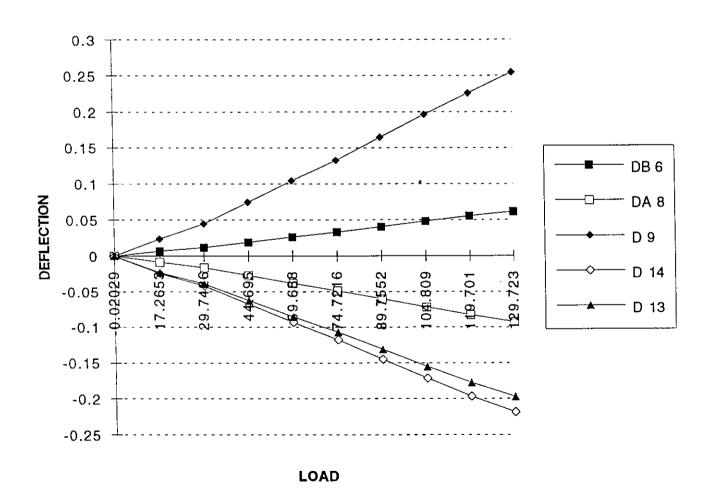
### R13 GRAPH 0.0INCG

# AS4/3501-6 [(0/30)6,(0/-30)6]



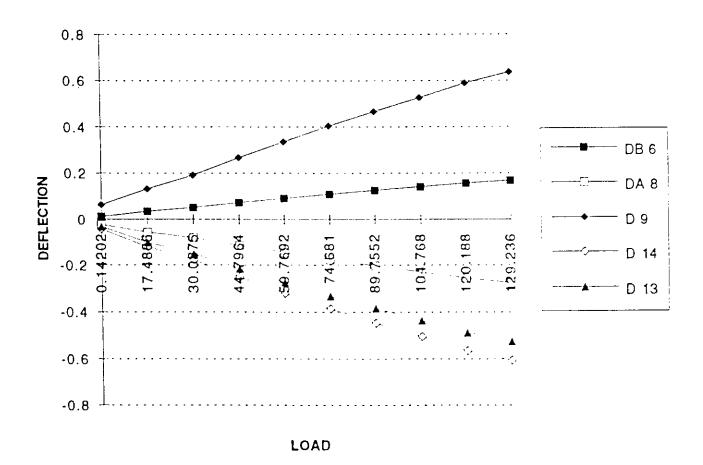
Graph 2

# AS4/3501-6 [(0/30)6,(0/-30)6]



Graph 3

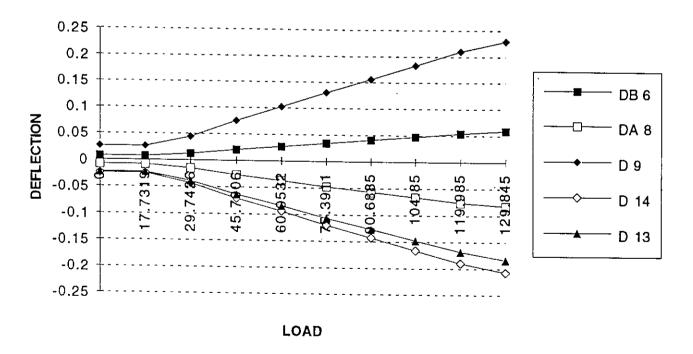
# IM7/8552 [(0/45)6,(0/-45)6]



Graph 4

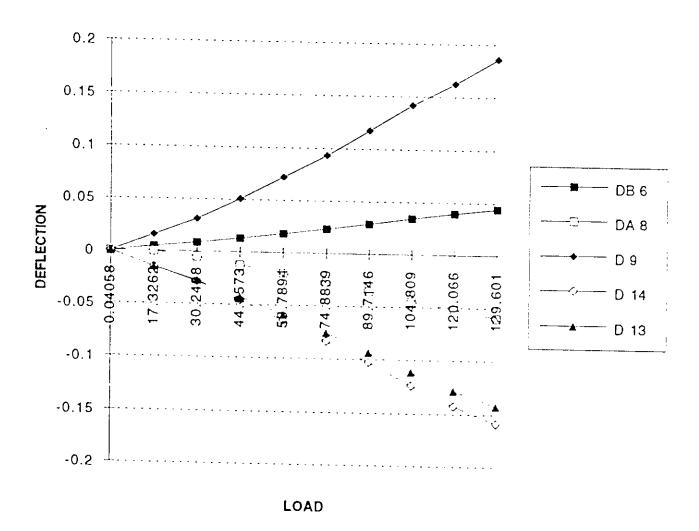
## AS4/3501-6 1.5 IN GC GRAPH

# AS4/3501-6 [0/60]12



Graph 5

# IM7/8552 [(0/75)6,(0/-75)6]



Graph 6

PT CDDF COMPBEAM S/N 20 RUN 19			
	" L 1"	" DB 8"	" DB 7"
SCAN"DELTA MINUTES	" LBS "	" INCH "	" INCH "
10 0.075000	0	-0.00003	-0.00029
17 0.816650	17.73192	-0.00438	0.00036
25 1.366633	29.74255	-0.00987	0.0004
33 1.816633	45.79056	-0.01885	0.00129
41 2.149950	60.05319	-0.02773	0.00212
49 2.574950	75.39111	-0.03662	0.00287
57 3.116600	90.68845	-0.04752	0.00423
65 3.591583	104.84964	-0.06319	0.00425
73 4.108250	119.98465	-0.07913	0.00521
81 4.524900	129.84473	-0.0887	0.00526
89 5.033217	0.30432	-0.01173	-0.00067
97 5.891533	17.79279	-0.01073	0.001
105 6.341533	30.0063	-0.0179	0.00218
113 6.733183	45.24277	-0.02924	0.00377
121 7.258167	59.87059	-0.04203	0.00552
129 7.624833	76.02005	-0.05608	0.00776
137 8.166483	90.20151	-0.06726	0.00881
145 8.616467	105.01193	-0.08014	0.00987
153 9.091467	120.26868	-0.09277	0.01083
161 9.483117	131.2243	-0.1036	0.01133
169 10.049783	0.36519	-0.013	-0.00065
177 10.516433	17.83336	-0.01542	0.00246
185 11.008083	30.14832	-0.02274	0.00406
194 11.449750	44.99931	-0.03615	0.00654
201 12.033067	60.19521	-0.04989	0,00894
210 12.499717	75.26938	-0.06231	0.01102
226 13.274700	104.68732	-0.08884	0.01536
235 13.716367	120.83676	-0.10312	0.01685
241 14.316350	130.65625	-0.11419	0.01751
250 15.049667	0.3449	-0.01225	-0.00115
257 15.549650	17.79279	-0.01579	0.0033
265 16.024650	30.10774	-0.02581	0.00568
273 16.441300	45.03989	-0.04019	0.00939
282 16.899633	60.49953	-0.0544	0.01218
290 17.507950	74.98535	-0.06911	0.01525
298 17.949600	89.75519	-0.08329	0.01814
305 18.357933	104.76846	-0.0974	0.02026
314 18.824583	119.98465	-0.1124	0.02229
321 19.141250	130.16931	-0.12092	0.02262
330 20.066217	0.30432	-0.01279	-0.00089
331 20.524550	17.73192	-0.01868	0.00485
346 21.041200	29.88457	-0.02853	0.00717

44.95874	-0.04423	0.01114
59.83002	-0.05784	0.01467
74.98535	-0.07449	0.01823
89.91748	-0.08921	0.02145
105.05252	-0.10437	0.02332
119.74121	-0.11879	0.02575
129.92584	-0.12942	0.02698
0.36519	-0.01371	-0.00098
29.80342	-0.03241	0.00968
44.8573	-0.0484	0.01453
59.87059	-0.06498	0.01849
74.92447	-0.08143	0.02207
90.20151	-0.09703	0.02549
105.09308	-0.11432	0.02889
119.9035	-0.13006	0.03171
129.96643	-0.1412	0.03495
0.36519	-0.01459	-0.00074
17.81307	-0.02229	0.00681
29.84399	-0.0364	0.0119
44.91815	-0.05346	0.01703
59.95174	-0.07144	0.02253
74.96506	-0.08766	0.02635
89.99863	-0.10573	0.03084
104.8902	-0.12126	0.03331
119.74121	-0.13889	0.03703
129.64185	-0.15148	0.03842
	59.83002 74.98535 89.91748 105.05252 119.74121 129.92584 0.36519 29.80342 44.8573 59.87059 74.92447 90.20151 105.09308 119.9035 129.96643 0.36519 17.81307 29.84399 44.91815 59.95174 74.96506 89.99863 104.8902 119.74121	59.83002       -0.05784         74.98535       -0.07449         89.91748       -0.08921         105.05252       -0.10437         119.74121       -0.11879         129.92584       -0.12942         0.36519       -0.01371         29.80342       -0.03241         44.8573       -0.0484         59.87059       -0.06498         74.92447       -0.08143         90.20151       -0.09703         105.09308       -0.11432         119.9035       -0.13006         129.96643       -0.1412         0.36519       -0.01459         17.81307       -0.02229         29.84399       -0.0364         44.91815       -0.05346         59.95174       -0.07144         74.96506       -0.08766         89.99863       -0.10573         104.8902       -0.12126         119.74121       -0.13889

" DB 6"	" DB 11"	" DA 8"	" DA 7"	" DA 6"	" DA 11"
" INCH "	" INCH "	" INCH "	" INCH *	" INCH *	* INCH *
-0.00008	-0.00028	0.00002	-0.00007	-0.00013	-0.00011
0.00689	0.00122	-0.00676	-0.00012	0.00738	-0.00115
0.01142	0.00235	-0.01257	-0.0001	0.01311	-0.00243
0.01897	0.00485	-0.02238	-0.00013	0.0219	-0.00243
0.02542	0.00687	-0.03193	-0.0001	0.03001	-0.00408
0.032	0.00922	-0.04071	-0.0001	0.03854	-0.00299
0.04023	0.01242	-0.05194	0.00023	0.04998	-0.00101
0.05155	0.01261	-0.06814	0.00021	0.06578	-0.00221
0.06194	0.01492	-0.08356	0.00074	0.08113	-0.00144
0.06822	0.01539	-0.09245	0.00178	0.09037	-0.00052
0.00293	-0.00024	-0.00846	0.00251	0.00613	0.00183
0.01259	0.00471	-0.01544	-0.00086	0.01282	-0.00677
0.02014	0.00776	-0.02517	-0.00291	0.02049	-0.01112
0.03057	0.01134	-0.03949	-0.00499	0.03173	-0.01602
0.04172	0.01609	-0.05513	-0.00694	0.04436	
0.05328	0.02047	-0.07346	-0.0091	0.05826	
0.06227	0.02348	-0.08434	-0.00918	0.06948	<del> </del>
0.07276	0.0264	-0.09908	-0.00912	0.08208	<del></del>
0.0822	0.02885	-0.11244	-0.00915	0.09475	-0.0283
0.08972	0.03035	-0.1235	-0.00913	0.1048	-0.02879
0.00467	-0.00019	-0.01266	0.00094	0.00769	-0.00075
0.01742	0.00748	-0.02275	-0.00477	0.01684	-0.01262
0.02667	0.01186	-0.03478	-0.0077	0.02543	-0.01983
0.03942	0.0175	-0.05317	-0.01115	0.03819	-0.02789
0.0523	0.02334	-0.07193	-0.01473	0.05214	-0.03498
0.06431	0.02847	-0.08856	-0.01703	0.06474	-0.04068
0.08708		-0.12059	-0.02045	0.09039	-0.04911
0.09849		-0.13644	-0.02094	0.10423	-0.05255
0.10644	0.04282	-0.14795	-0.02152	0.11454	-0.05421
0.00472	-0.00118	-0.00739	0.00041	0.00725	-0.00207
0.02034	<del></del>	-0.02621	-0.00742	0.018	-0.01834
0.03194	<del></del>	-0.0426		0.02843	-0.02845
0.04688			-0.01726	0.04202	-0.03935
0.06128		<del></del>	<del> </del>	+ <del></del>	-0.04858
0.0753	<del></del>		-0.02565	0.0715	-0.05701
0.08813	<del>                                     </del>	<del></del>	<del></del>	0.08517	-0.06404
0.09994		<del></del>		0.09889	-0.06905
0.11233		<del></del>		·····	-0.0747
0.1192					•
0.00636	<del></del>	-0.01483	<del></del>	<del></del>	
0.02466			<del></del>	<del></del>	1
0.03681	0.01859	-0.04956	-0.01652	0.03074	-0.03668

Chart 3

0.0534	0.02828	-0.07406	-0.02301	0.04627	-0.05064
0.06856	0.036	-0.09511	-0.02776	0.06035	-0.06191
0.08386	0.04339	-0.1169	-0.03252	0.07567	-0.07231
0.09771	0.04979	-0.13726	-0.03662	0.09026	-0.08143
0.11087	0.05435	-0.15655	-0.03936	0.10458	-0.08909
0.12367	0.05915	-0.17523	-0.04177	0.11877	-0.09608
0.1322	0.06174	-0.18744	-0.0436	0.12886	-0.10008
0.0077	-0.00042	-0.01831	-0.00466	0.00925	-0.00789
0.04277	0.02292	-0.05831	-0.02104	0.03473	-0.04557
0.06108	0.03369	-0.08503	-0.02899	0.05048	-0.06288
0.07845	0.04273	-0.10936	-0.03582	0.06626	-0.07735
0.09453	0.05106	-0.13374	-0.04128	0.08195	-0.09089
0.10966	0.05807	-0.15614	-0.04623	0.09757	-0.10199
0.12502	0.0647	-0.17883	-0.05067	0.11326	-0.11321
0.13873	0.06955	-0.19874	-0.05447	0.12833	-0.1214
0.1478	0.07346	-0.21218	-0.05794	0.13882	-0.12655
0.00887	-0.00024	-0.01974	-0.00745	0.01071	-0.00983
0.03099	0.01671	-0.04203	-0.01713	0.02415	-0.03679
0.0482	0.02786	-0.06691	-0.02613	0.03777	-0.05518
0.06921	0.04009	-0.09724	-0.03566	0.05498	-0.07661
0.08806	0.05073	-0.1255	-0.04435	0.07197	-0.0941
0.10458	0.05882	-0.14938	-0.05059	0.08733	-0.10939
0.12177	0.06743	-0.17597	-0.05695	0.10409	-0.12397
0.13643	0.07284	-0.19669	-0.06141	0.11908	-0.13583
0.15214	0.08	-0.22037	-0.06686	0.13516	-0.14749
0.1625	0.08263	-0.2352	-0.06897	0.14649	-0.15392

-	D 9"	u	D 14"	u	D 13"	11	D 12"		D 10"
<b>*</b>	INCH *	н	INCH "	п	INCH "	#		N	D 10"
	0		0.00019						INCH *
	0.02193		-0.02177		-0.00005		-0.0001		-0.00005
	0.03925		-0.02177		-0.02025		-0.01786		0.01736
	0.06991		-0.03713		-0.03398		-0.03048		0.0297
	0.0965				-0.05634		-0.04965		0.04894
<b>—</b>	0.12187		-0.0863 -0.10924		-0.07592		-0.06687		0.06615
	0.15467		-0.13743		-0.09535		-0.08512		0.08411
	0.20454			<u></u>	-0.11916		-0.10826		0.10769
-	0.24951		-0.17455		-0.15203		-0.13966		0.13842
<u> </u>	0.2761		-0.21051	·	-0.18245		-0.16916		0.16842
<u> </u>	0.01683		-0.23112		-0.20069		-0.18804		0.18711
	0.01663		-0.0105		-0.00926		-0.01037		0.0113
-	0.07564		-0.03985		-0.03671		-0.02891		0.02994
-	0.07564		-0.06628		-0.05922		-0.04598	<del></del>	0.04563
-	0.16219		-0.10205		-0.08964		-0.07025		0.06951
	0.16219		-0.14015		-0.12208		-0.09671		0.09595
	0.21894		-0.18077		-0.15587		-0.12469		0.12353
-	0.29114		-0.21012		-0.18245		-0.14842		0.14706
	0.32937	 	-0.24589		-0.21345		-0.17464		0.17315
$\vdash$	0.36158		-0.27835		-0.24186		-0.20066		0.19856
-			-0.30362		-0.26384		-0.22082	<u> </u>	0.21859
-	0.02634 0.06472		-0.01711		-0.01473		-0.01375		0.01475
			-0.05695		-0.0512		-0.03713		0.03779
	0.10242 0.15292	<u> </u>	-0.08883		-0.07856		-0.05645		0.05668
-			-0.13237		-0.11556		-0.08385		0.08391
	0.20794	<u> </u>	-0.17689		-0.15385		-0.11315		0.11277
	0.25601	_	-0.21829		-0.18888		-0.14093	<del>                                     </del>	0.13951
	0.34955		-0.29682		-0.25664	<u> </u>	-0.19558		0.19274
	0.39564		-0.33511		-0.29014	<u> </u>	-0.22454		0.22086
	0.42786	+	-0.36193		-0.31384		-0.24538		0.24148
-	0.01596	-	-0.01283		-0.0132	-	-0.01247		0.0147
-	0.07748	<del></del>	-0.06784	ļ	-0.0596	<del></del>	-0.0407		0.04168
	0.12318		-0.10691		-0.09348	<u></u>	-0.06364		0.06379
-	0.1831		-0.15822	<u> </u>	-0.13729		-0.09432	ļ	0.09348
	0.24262	-	-0.20818		-0.18		-0.12577		0.12367
	0.29958	<del> </del>	-0.25639		-0.22142		<i>-</i> 0.15703	<del>!</del>	0.15406
-	0.35169	+	-0.30051	<u> </u>	-0.25962	<u> </u>	-0.18658	<del>1</del>	0.18223
_	0.40224	<del></del>	-0.3425		-0.29522	<del></del>	-0.21534	ļ	0.20981
-	0.45405	+	-0.38565	<u> </u>	-0.33285	-	-0.24577	ļ	0.23871
-	0.48142	+	-0.40781	<u> </u>	-0.35329	<del></del>	-0.26348		0.25608
-	0.03508	-	-0.02294		-0.01795	-	-0.01355	ļ	0.01529
_	0.10251		-0.08339		-0.0716	<del></del>	-0.04603		0.04622
	0.1439	<u> </u>	-0.12363	<u> </u>	<u>-0.10711</u>		-0.06966	L	0.06941

0.21143	-0.18174	-0.15702	-0.10366	0.10197
0.2727	-0.23423	-0.20169	-0.13541	0.13265
0.33374	-0.28749	-0.2481	-0.16887	0.16398
0.39244	-0.3355	-0.28975	-0.20086	0.19353
0.44707	-0.38176	-0.33016	-0.23217	0.22332
0.4983	-0.42725	-0.36893	-0.26279	0.25253
0.53478	-0.4564	-0.39466	-0.28432	0.27325
0.04303	-0.02682	-0.02159	-0.01649	0.02146
0.16815	-0.14481	-0.12477	-0.07861	0.0815
0.24253	-0.20857	-0.17957	-0.11491	0.11534
0.31579	-0.26902	-0.23121	-0.15042	0.14829
0.38245	-0.32695	-0.2815	-0.18579	0.18085
0.44319	-0.38021	-0.32805	-0.21994	0.21267
0.50587	-0.43502	-0.37565	-0.25506	0.24463
0.56137	-0.48323	-0.41759	-0.28803	0.27512
0.59999	-0.51549	-0.44562	-0.31112	0.29633
0.04691	-0.03013	-0.02438	-0.01903	0.02314
0.12163	-0.10438	-0.08955	-0.05523	0.05772
0.18916	-0.16406	-0.14085	-0.08654	0.08761
0.27474	-0.2385	-0.20443	-0.12699	0.12422
0.35256	-0.30615	-0.26288	-0.16574	0.15954
0.41932	-0.36466	-0.31365	-0.20125	0.19165
0.49267	-0.42841	-0.36778	-0.23951	0.22589
0.55244	-0.47934	-0.41376	-0.27346	0.25608
0.61571	-0.53649	-0.46309	-0.31063	0.28923
0.65918	-0.57264	-0.49457	-0.33578	0.31212

**SECTION II** 

Test Strain Data

n	1T1009"	" 1T1010"	" 1T1011"	" 1T1012"	" 1T1013"	" 2T1009"
н	MST "	" MST "	" MST "	" MST "	" MST "	" MST "
	0	-0.26075	-1.04522	-1.04236	-1.04527	-1.03109
	-1.82052	-7.04018	-15.15401	-3.90876	-12.02065	16.75423
	-0.26009	-8.3439	-21.4245	-3.38757	-15.15646	27.32236
	1.0403	-11.47288	-31.87544	0.2607	-13.58858	41.75674
	2.08057	-14.60185	-40.49747	0.2607	-17.24705	53.35596
	2.86079	-17.99157	-47.02927	0	-17.76966	56.44922
	1.56043	-22.16352	-56.95758	-0.26048	-20.64416	66.24393
	1.0403	-26.07473	-65.3183	-1.82403	-26.13184	72.9458
	1.0403	-30.50742	-71.58899	-2.08472	-27.43847	76.55438
	0.78021	-31.81113	-74.72424	-3.12688	-28.74505	76.03903
	2.08057	1.56446	-1.56783	2.34521	3.1358	0.77301
	-1.56044	-5.73647	-29.00151	6.77516	-2.09054	42.01483
	-4.68134	-9.90842	-41.80379	5.47231	-13.58858	64.18216
	-5.46156	-14.60185	-57.2189	8.3387	-17.50835	86.86484
	-5.72161	-18.25231	-70.80508	11.72628	-18.81492	108.00107
	-5.20148	-22.42426	-82.56235	14.59268	-21.16682	125.52875
	-5.9817	-25.0317	-93.79721	17.45908	-21.95078	140.9942
	-8.32235	-29.98594	-106.07709	18.76192	-25.60925	155.94434
	-8.84251	-34.41862	-116.00543	20.32547	-26.6545	167.80121
	-9.62273	-37.54759	-121.49219	21.62833	-25.60925	173.98737
	1.0403	1.04297	-3.13545	2.8664	4.96503	2.31984
	-4.68134	-5.99721	-35.7946	8.59919	-4.70374	56.96455
	-7.54213	-10.16916	-53.56104	9.64156	-14.89515	86.34909
	-8.06229	-16.16631	-74.72424	16.6772	-15.41781	120.37329
	-12.22346	-21.12055	-95.88724	21.36784	-19.33758	155.42859
	-13.52382	-25.81399	-110.51862	25.01591	-21.95078	176.56488
	-16.6447	-30.76817	-126.71747	28.14279	-25.87054	200.79437
	-19.24542	-35.98308	-138.99731	30.7487	-28.74505	219.35309
	-22.88646	-41.4588	-151.7998	33.09392	-30.83558	234.30322
	-23.92676	-44.06628	-161.20551	33.87558	-29.529	244.61328
	-8.58243	-3.91121	-6.532	-6.25397	-3.91977	-4.63969
	-15.08426	-13.03739	-48.33557	-0.52118	-15.94042	63.66641
	-18.98537	-18.51305	-71.3277	5.21162	-22.73473	100.78387
	-21.84616	-25.0317	-97.45505	13.02914	-24.56393	146.14923
	-24.4469	-29.98594	-120.18567	20.84666	-27.69978	186.87531
	-27.82787	-34.94011	-141.08752	27.8823	-29.00634	220.89954
_	-28.86815	-39.89429	-154.41241	34.13628	-28.48374	244.61328
	-31.72896	-43.8055	-170.35016	37.26315	-32.9261	268.84277
	-35.63007	-50.32413	-185.24268	41.43262	-36.58456	291.01013
	-39.01102	-53.71387	-195.43231	43.77783	-36.84592	305.44458
	-16.90477	-8.60464	-12.54118	-15.63483	-9.40746	-6.95953
	-23.40663	-18.25231	-61.92175	-0.78167	-17.76966	<u> </u>
<u> </u>	-26.78757	-23.98873	-88.0491	5.47231	-26.91581	127.07516
	-30.42859	-29.98594	-118.35675	15.63504	-29.7903	178.62701

Chart 1

### **STRAINS**

-35.36998	-36.50456	-144.74542	25.27639	-29.79031	226.57031
39.53116	41.4588	-167.4762	33.6151	-31.09689	264.71887
-42.65205	-47.71671	-187.85547	41.43262	-35.278	301.83594
-46.033	-52.93167	-207.45105	48.72894	-37.36853	337.40662
-50.19417	-58.66803	-222.86597	53.41937	-39.19777	361.63611
-53.31505 <sup>1</sup>	-62.05774	-233.31689	54.46175	-41.2883	376.07043
-23.40663	-14.60185	-18.55057	-20.06478		-7.73294
-30.42859	-23.20649	-71.58899	-6.51446	-27.43847	
-34.32968	-28.42142	-105.55447	6.25397	-29.79031	152.33575
-39.53116	-35.46159	-141.34888 <sup>1</sup>	20.58596	-32.4035	218.83734
43.69232	-41.198	-170.87274	33.09392	-31.88085	273.48242
-50.45424	-48.23819	-200.13538	44.03831	-35.278	328.12756
-54.61543	-53.45316	-221.82074	52.11632	-37.62984	366.7915
-57.73631	-59.45032	-240.37134	60.9762	-37.89119	
-62.41762	-64.66528	-259.70532	68.79373	-39.98172	435.87061
-64.75827	-67.27274	-270.67896	74.00534	-39.72043	455.46045
-24.70699	-15.12334	-19.07297	-21.10714	-11.49805	-8.50595
-32.76926	-24.77096	-82.56235	-1.82403	-27.17712	111.60968
-37.45058	-30.50742	-120.18567	12.76844	-29.79031	183.78204
-40.57146	-37.02606	-160.16052	32.31224	-28.48374	261.62561
-45.77293	-41.71954	-195.17102	48.46826	-30.05161	331.22046
-49.67403	-47.71671	-221.55988	63.06093	-30.05161	386.38098
-55.13557	-53.19235	-247.16443	74.26584 <sup>1</sup>	-30.051€1	436.38635
-59.81688	-57.88586	-269.89502	85.47092		
-65.79855	-63.10074	-290.27466	93.80963	-33.44876	518.354
-68.91946	-66.49045	-304.38306	100.06342	-34.23273	543.61426
<del>-</del>				97,20270	010.01720

#### **STRAINS**

" 2T1010"	" 2T1011"	" 2T1012"	"_2T1013"	" 3T1009"	" 3T1010"
" MST "	" MST "	" MST "	" MST "	" MST "	" MST "
-1.54683	1.02696	-4.36252	-3.86532	0	-2.58828
4.64089	-18.99861	-4.10595	-6.18445	19.06746	10.87062
6.18772	-33.11916	-2.82282	-5.15372	29.77654	18.37659
6.96113	-50.3205	21.29935	10.82273	43.09766	25.6237
8.76603	-70.08929	29.76782	18.29559	54.85156	32.09435
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73.99417	-37.22691	45.16496	13.14192	90.37445	72.47113
86.3692	-51.0907	64.66806	23.19164	111.27023	86.96536
96.16632	-67.26517	80.83499	33.7567	130.07648	100.16541
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113.18265	-119.38281	126.76984	61.84436	174.74127	128.37738
112.92459	-132.73322	144.47662	73.95554	180.48773	131.4834
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240.80286	-32.60562	141.65375	42.51802	230.63776	202.1427
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# STRAINS

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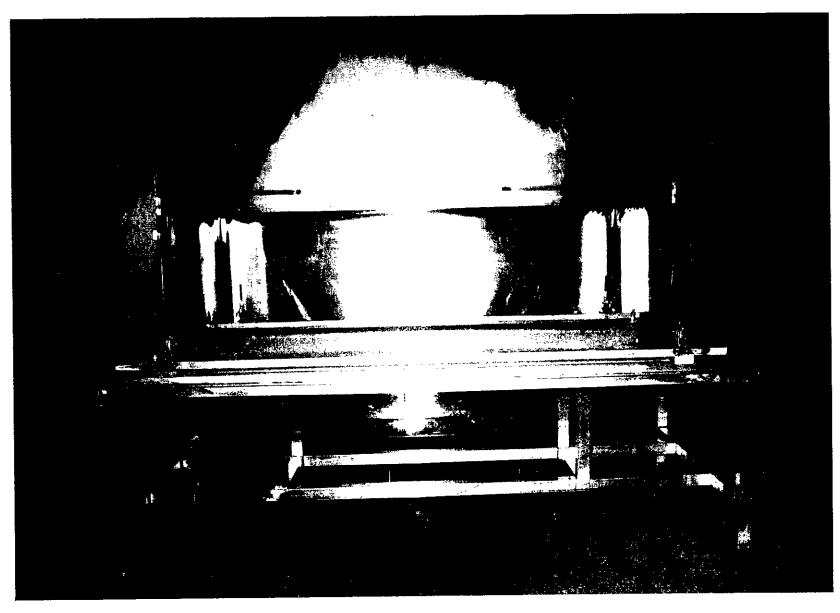
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" MST "	" MST "	"	MST "	" LBS "
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-101.63284		<del>-</del>	33.97525	<u> </u>
-1.82413	-3.90581		1.56803	· · · · · · · · · · · · · · · · · · ·
-65.67041	-2.34348		4.18168	· · · · · · · · · · · · · · · · · · ·
-98.50565	-9.11354		3.65887	·
-128.4743	-1.82271		8.36316	<del> </del>

## STEAINS

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40.09969	22.21473	75.02591
48.43207	26.1349	89.75516
54.94174	31.3618	104.64673
57.806	34.75937	119.49771
60.14949	37.89563	130.29108
	22.91415 18.48758 20.83106 22.39338 21.61222 1.56232 -13.54012 -5.9889 4.68697 22.39338 23.69531 28.64267 39.83931 38.0166 42.18279 -0.52077 -7.81161 -2.0831 19.52912 27.60112 40.09969 48.43207 54.94174 57.806	22.91415       19.07847         18.48758       22.21473         20.83106       26.91901         22.39338       30.31638         21.61222       32.14592         1.56232       1.56803         -13.54012       0         -5.9889       2.87496         4.68697       8.10186         22.39338       14.63548         23.69531       16.98763         28.64267       21.95322         39.83931       27.18032         38.0166       30.8392         42.18279       34.23676         -0.52077       0.52281         -7.81161       1.04542         -2.0831       3.92017         19.52912       10.97661         27.60112       15.9422         40.09969       22.21473         48.43207       26.1349         54.94174       31.3618         57.806       34.75937

# SECTION III

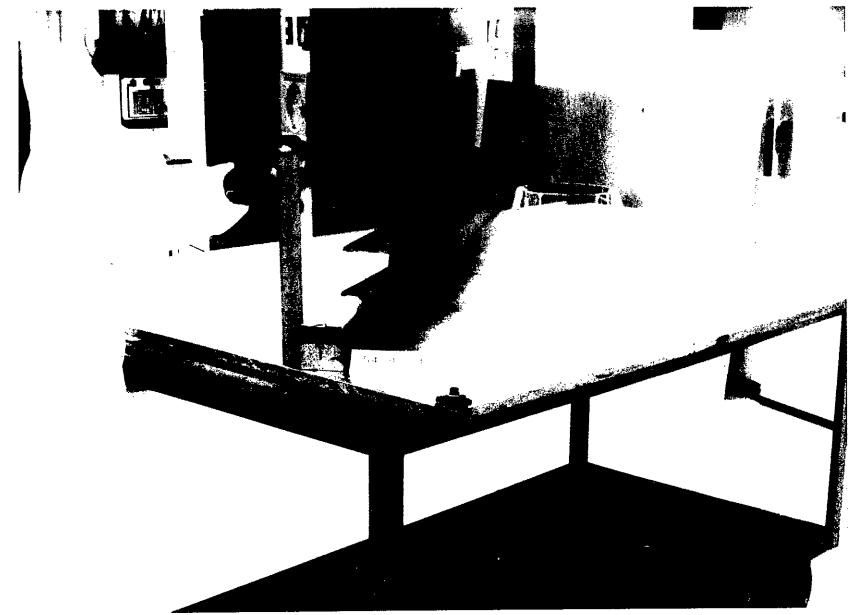
Manufacturing and Testing Photographs



Hot-drape forming "oven."

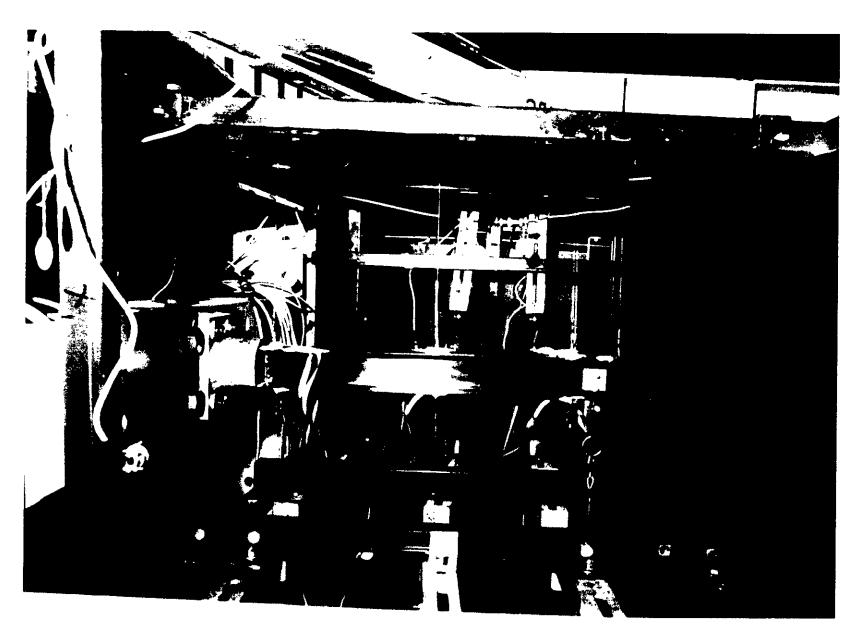


Laminate placed on tool ready for hot-drape forming.

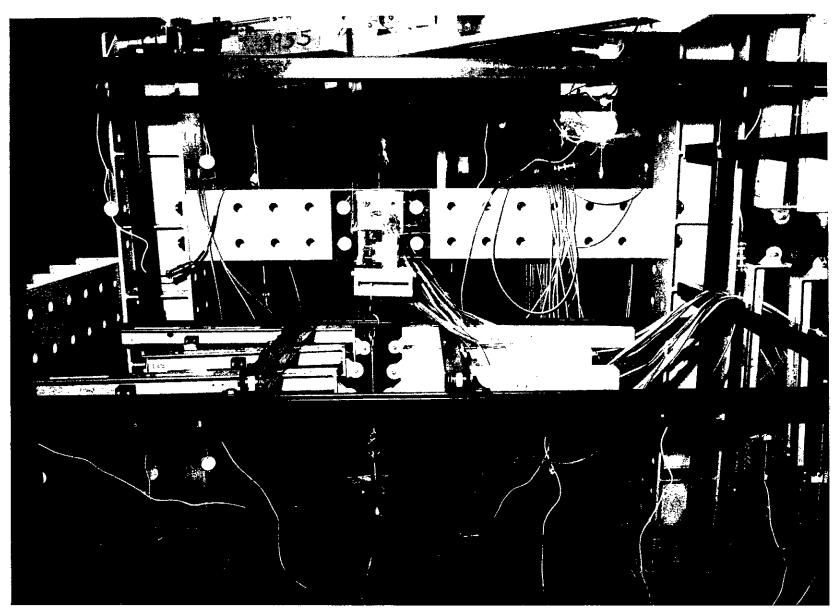


C-channel upon removal from tool after autoclave cure.

Notice warpage of beam along its length.



Fully instrumented beam C-channel prior to testing.



Front view of test configuration. Slide block allows load application point to be changed.



C-channel with load being applied. No rotation has occurred at this point of the test.



Test being performed on L-angle. Beam has started to rotate.

#### REPORT DOCUMENTATION PAGE

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13. ABSTRACT (Maximum 200 words)					
Presented in this repo deformations occurring in op- manufactured using both har transverse tip load was appli allowed the tip load to be ap- center. Charts are included in load application points.	nen-section composite nd layup and the innoved at the free end of the plied at various location	ative "hot-drape forming e cantilevered open-secti ons along the plane of and	L channels were "techniques. A on beams. The test setup I at the beam's shear		
composite beam can be alter observed that the choice of t deformation, as expected, an composite beam's true shear further investigation into the beams.	ed, if not completely c he material system doe id the material affects t center. The results fro	is not have an effect on the he location of an unsymment in this study have provid	ate layup. Also, it was ne amount of metric open section led a foundation for a open-section composite		
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antisymmetric laminate layup, hot-drape forming, torsion/bending coupling.			. 54 16. PRICE CODE		
composite beams	10. FMCE CODE				

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